

CODE

- 1. ALL CONSTRUCTION SHALL FOLLOW LOCAL STATE BUILDING CODE, MANUFACTURES'
 SPECIFICATIONS, AND WELL KNOWN INDUSTRY STANDARDS. IF ANY QUESTIONS SHALL ARISE, THE
 DESIGNER OR ENGINEER ON RECORD SHALL BE CONTACTED.
- 2. INTERNATIONAL RESIDENTIAL CODE 2009 (IBC 2009) AND THE REFERENCED STANDARDS INCLUDED THEREIN. AHJ = AUTHORITY HAVING JURISDICTION.

A. NUMBER OF UNITS:

____ (1 OR 2) ___2_ (MAX. 3)

B. NUMBER OF STORIES,

DESIGN LOADS:

1. UNIFORM FLOOR LIVE LOAD (NON-BEDROOM).

A. NON -BEDROOM B. BEDROOM 40PSF 30PSF

C. ATTIC 20PSF
2. UNIFORM FLOOR DEAD LOAD: 10PSF
3. ROOF SNOW LOAD (AH.I):

3. ROOF SNOW LOAD (AHJ):

A. GROUND SNOW LOAD: 60 PSF (TOWN, STATE SPECIFIC)

4. WIND DESIGN:

4. WIND DESIG

- A. EXPOSURE CATEGORY ______ (A-D, R301.2.1.4) (B IS NORMAL)

 B. WIND SPEED ZONE (AHJ) _____ (90 -120, MOST OF NORTHERN AND WESTERN
- B. NIND SHEED ZONE (ARL) ______ (90 -120, MOST OF NORTHERN AND WESTERN MIND MAS = 100, NH COAST, BOSTON AND SOUTH = 110, CAPE COD AND ISLANDS = 120, R301.2.4)

C. TOPOGRAPHIC EFFECTS (AHJ):____ (YES/NO)

- 5. SEISMIC
 - A. DESIGN CATEGORY (AH.) _____(AE, R301.2.2), (C FOR MOST OF SOUTH AND CENTRAL NH AND B FOR MA)
- 6. DAMAGE:
 - A. WEATHERING: SEVERE (CONCRETE) (R301.2(3))
 - B. TERMITE INFESTATION PROBABILITY:_____
- SLIGHT (NORTHERN NH), MODERATE (SOUTHERN NH), HEAVY (MA)
- 7. DESIGN FROST DEPTH OF __4_FEET BELOW FINISHED GRADE (4' IS TYPICAL; VERIFY AS NEEDED WITH AHJ)
- 8. WINTER DESIGN TEMP: NH: O DEG. F., MA IO DEG. F. (PER 301.2(1))
- 4. FLOOD HAZARD (AHJ): ___

GENERAL NOTES:

- I. THESE DRAWINGS REPRESENT AN OVERALL DESIGN CONCEPT. THEY ARE PREPARED WITH THE INTENT TO DEMONSTRATE THE OVERALL DESIGN ARRANGEMENT AND METHODS OF ASSEMBLY TO THE VARIOUS COMPONENTS. THE DRAWINGS DO NOT INDICATE EXTENSIVE DETAILS. THE CONTRACTOR SHALL HAVE REVIEWED THESE PLANS, SEEN THE SUBJECT PROPERTY, AND BE CAPABLE OF EXECUTING THE DETAIL WORK AS NECESSARY TO ACHIEVE THE INTENDED RESULT, IN A MANNER CONSISTENT WITH QUALITY WORKMANSHIP WITHIN THE REGION.
- 2. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL STATE AND LOCAL CODES, REGULATIONS AND FHAVYA MPS.
- 3. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT SITE BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO ALTERNATIVE DESIGNS INC. FOR JUSTIFICATION AND OR CORRECTION BEFORE PROCEEDING WITH WORK.

- 4. THE OWNER AND CONTRACTOR SHALL HOLD HARMLESS THE DESIGNER FROM AND AGAINST ALL CLAIMS, DAMAGES, LOSSES AND EXPENSES (INCLUDING LEGAL FEES) ARISING OUT OF OR RESULTING FROM THE PERFORMANCE OF THE WORK BY THE CONTRACTOR.
- 5. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
 6. ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD BY THE CONTRACTOR.
 ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER OR STRUCTURAL
 ENGINEER BEFORE PROCEEDING.
- 7. IN THE EVENT OF A CONFLICT BETWEEN PLANS, SPECIFICATIONS, AND DETAILS, THE DESIGNER OR STRUCTURAL ENGINEER SHALL BE NOTIFIED IMPEDIATELY FOR CONSULTATION. IF CONDITIONS AT THE SITE ARE DIFFERENT THAN SHOWN, THE DESIGNER OR STRUCTURAL ENGINEER SHALL BE NOTIFIED BEFORE ANY WORK IS PROCEEDED WITH.
- 6. ALTERNATIVE DESIGN ASSUMES NO LIABILITY AS A RESULT OF ANY CHANGES OR NON CONFORMANCE WITH THESE PLANS EXCEPT UPON THE WRITTEN APPROVAL OF THE DESIGNER OR ENGINEER ON RECORD.
- ALTERNATIVE DESIGN ASSUMES NO LIABILITY FOR WORK PERFORMED WITHOUT AN ACCEPTABLE PROGRAM OF TESTING AND INSPECTION AS APPROVED BY THE ENGINEER ON RECORD.
- IO.REPRODUCTION OF DESIGNER PLANS AND STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
- II. SECTIONS, DETAILS, NOTES, METHODS, OR MATERIALS SHOWN AND/OR NOTED ON ANY PLAN, SECTION, OR ELEVATION SHALL APPLY TO ALL OTHER SIMILAR LOCATIONS UNLESS NOTED OTHERWISE.
- 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED DURING CONSTRUCTION. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE REMOVED AFTER CONSTRUCTION AND ARE THE RESPONSIBILITY OF THE CONTRACTOR.

intractor to check 4 werity all mensons 4 structural members since construction.

Construction shall be in strict majorace with The State of monitorice with The State of with Hompshire or Moscoatheettis Millary Cookes, witchew applicable.

REVISIONS

14-044 FEB 2014 SHEET 1 OF 8

Al

FOUNDATIONS:

- I. FOUNDATIONS CONSIST OF CONTINUOUS FOOTINGS ASSUMED TO BEAR ON COMPACTED STRUCTURAL FILL PLACED ON UNDISTURBED NATURAL SOIL HAVING AN ASSUMED ALLOWABLE BEARING PRESSURE OF 2500 PSF (TO BE VERIFIED BY BUILDER). IF THE SOIL AT BEARING DEPTH IS DISTURBED OR THE ACTUAL ALLOWABLE BEARING PRESSURE IS LESS THAN 2500 PSF, THEN A QUALIFIED GEOTECHNICAL BYGINEER SHALL BE CONSULTED.
- 2. UNLESS OTHERWISE NOTED, FOOTINGS SHALL BE CENTERED UNDER SUPPORTED MEMBERS.
- 3. THE BOTTOM PERIMETER FOUNDATIONS SHALL BE DESIGN FROST DEPTH BELOW FINISHED GRADE.
- 4. THE BOTTOM 3 INCHES OF FOOTING EXCAVATIONS SHALL BE FINISHED BY HAND SHOVEL
- 5. FINISH EXTERIOR GRADE SHALL BE AT LEAST 8" BELOW TOP OF FOUNDATION WALL.
- 6. PLACE BACKILL SIMULTANEOUSLY ON BOTH SIDES OF WALLS TO THE GRADES INDICATED.
- 7. UNBRACED/UNBALANCED FOUNDATION WALLS: MAXIMIM UNBALANCED FILL: 24" WITHOUT DESIGN/ENGINEER INPUT/APPROVAL. (EXAMPLESARAGE SLAB ON GRADE WHERE BACKFILL WILL BE MORE THAN 24" BELOW TOP OF SLAB) (SEE R404.1.2) (ENGINEER DESIGN REQUIRED WHEN >48")
- WE RECOMMEND THAT WALKOUT AND KNEEWALL STYLE BASEMENTS BE REVIEWED. (IE. WHENEVER PERIMETER FOUNDATION WALLS ARE NOT FULL HEIGHT).
- PROVIDE FORMMORK FOR ALL FOOTINGS, WALLS, AND PIERS. EARTH FORMED FOUNDATIONS ARE NOT ALLOWED.
- IOSUB-SOIL SHALL HAVE 3/4 " MAXIMUM AGGREGATE WITHIN 12" OF SLAB ON GRADE
- ANCHOR BOLTS: 1/2" X 9" (MIN. 7" EMBEDMENT) @ 4" OC AND BETWEEN 6-12" OF EACH END. (R403.16)
- 12.DAMP PROOFINGLWAYS REQUIRED BELOW GRADE WHEN INTERIOR SPACE IS CREATED (PER R406)
- IS WATERPROOFING REQUIRED WHEN INTERIOR SPACE CREATED AND HIGH WATER TABLE OR OTHER CONDITIONS. (PER R406)

CONCRETE

- CONCRETE SHALL BE A MIX DESIGNED FOR ULTIMATE STRENGTH IN ACCORDANCE WITH ACI 211.1 TO ACHIEVE THE DESIRED COMPRESSIVE STRENGTH. STANDARD MINIMUM 3,000 PSI FOR FOOTINGS AND INTERIOR FLOOR, 3,500 PSI FOR WALLS AND GARAGE SLAB. (R402.2)
- 2. CONCRETE SHALL NOT BE CAST IN WATER OR ON FROZEN GROUND, CONCRETE SHALL NOT BE EXPOSED TO WATER (I.E. RAIN) DURING SETTING PERIOD.
- 3. CONCRETE FLOORS SHALL BE PLACED OVER MIN. 4" THICK POROUS LAYER (SUCH AS CRUSHED STONE) WITH DRAINAGE AND APPROVED VAPOR BARRIER. (R405.2.2)
- 4. TOP OF FOUNDATION WALLS AND SLABS SHALL BE SMOOTH AND LEVEL.
- NO PIPE GREATER THAN 4" DIAMETER SHALL PASS THROUGH CONCRETE WITHOUT PERMISSION OF THE STRUCTURAL ENGINEER. PIPE SLEEVES SHALL BE PROVIDED AND SPACED A MINIMUM THREE DIAMETERS APART.
- 6. KEYS SHALL BE 2"X4", WITH BEVELED SIDES, UNLESS OTHERWISE NOTED
- CONSTRUCTION JOINTS SHALL BE FORMED WITH A KEY, AND REINFORCING SHALL BE LAPPED TO DEVELOP THE FULL TENSION CAPACITY OF THE (SMALLER) BAR.
- 8, EXPOSED CONCRETE SHALL BE RUBBED IMMEDIATELY AFTER REMOVAL OF FORMS AND SNAP TIES REMOVED TO FLUSH.
- 9. OPENINGS IN CONCRETE WALLS SHALL BE LOCATED, SIZED, AND REINFORCED (WITH THE EXCEPTION OF SHALL OPENINGS AND/OR SLEEVES OF A SIZE THAT WILL NOT DISPLACE OR INTERRUPT THE CONTINUITY OF THE REINFORCING) AS SHOWN ON RESPECTIVE DETAILS. ANY ALTERATIONS REQUIRE APPROVAL OF THE STRUCTURAL ENGINEER.
- IO. DO NOT BACKFILL FOUNDATION WALLS UNTIL THE CONCRETE HAS BEEN IN PLACE FOR SEVEN (1)
 DAYS AND ATTAINED 15% OF ITS DESIGN COMPRESSIVE STRENGTH, AND FLOOR DIAPHRAGMS
 ARE IN PLACE. (R404.1.1)

RENFORCING STEEL

- REINFORCING STEEL SHALL BE NEW STEEL BAR, FREE FROM LOOSE RUST AND SCALE, AND CONFORMING TO ASTM ASIS, SR 60.
- 2. STANDARD MINIMUM FOUNDATION FOOTING: 16" WIDE X 8" HIGH WITH NO REINFORCING.
- STANDARD MINIMUM VERTICAL FOUNDATION WALL REINFORCING FOR COMMON CONDITIONS:

WALL HEIGHT	MAX. BACKFILL	WALL THICKNESS	HORIZONTAL REINFORCING (R404.1.2)	VERTICAL REINFORGING
8'	7'	8*	#4 WITHIN I2" OF TOP AND #4 AT	#6 @ 36" OC
q'	8'	10 ^a	I #4 WITHIN I2" OF TOP AND #4 BARS AT THIRD HEIGHTS	#6 @ 30" OC

TABLE ABOVE ASSUMES BEST SOIL CLASS GW, GP, SM AND SP.

- 4. FLATWORK: WELDED WIRE FABRIC (WAF 6"X6" X NO. 10) RECOMMENDED IN ALL FLATWORK. IT SHALL CONFORM TO ASTM A185, LAP TWO SQUARES AT JOINTS AND TIE AT 3"-O" O.C. FURNISH WAF IN FLAT SHEETS.
- 5. PLAN CONTROL JOINTS AT 10-12' OC BOTH DIRECTIONS. WHF MUST NOT CROSS CONTROL JOINTS.
- 6. DECOUPLE FLATWORK FROM WALLS.
- WELDED WIRE FABRIC SHALL BE SUPPORTED ON CONCRETE BRICKS SP. AT 24" OC EACH DIRECTION ON GRADE, MELDED MIRE FABRIC SHALL BE SUPPORTED ON ELEVATED DECK WITH CONTINUOUS BOLSTERS LOCATED OVER JOISTS AND BEAMS.
- 8. CLEAR CONCRETE COVER OVER BARS SHALL BE IN ACCORDANCE WITH ACI 318.
- q. ACCESSORIES SHALL HAVE UPTIRNED LEGS AND BE PLASTIGDIPPED AFTER FABRICATION. ACCESSORIES FOR REINFORCING SHALL BE IN ACCORDANCE WITH THE MOST CURRENT ACI EDITION.
- IO. LAP REINFORCING TO DEVELOP THE FULL TENSION CAPACITY OF THE (SMALLER) BAR

- II. NO BARS SHALL BE CUT OR OMITTED IN THE FIELD BECAUSE OF SLEEVES, DUCT OPENINGS, OR RECESSES. BARS MAY BE MOVED ASIDE WITHOUT CHANGE IN LEVEL WITH THE PRIOR APPROVAL OF STRUCTURAL ENGINEER.
- 12. ANCHOR BOLT MATERIAL SHALL CONFORM TO ASTM A36, A301, OR BETTER, AND MEET IRC 2004 CODE.

MOOD

- WORK SHALL BE IN ACCORDANCE WITH THE AMERICAN WOOD COUNCIL, ANSI/AF4PA, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION 2012 (NDS)" INCLUDING "DESIGN VALUES FOR WOOD CONSTRUCTION", NATIONAL FOREST PROTECTION ASSOCIATION.
- ALL LIMBER SHALL BE NEW AND STRAIGHT AS DESCRIBED IN "STANDARD GRADING RILES FOR NORTHEASTERN LIMBER" BY NORTHEASTERN LIMBER MANIFACTURERS ASSOCIATION.
- NEW WOOD FOR STRUCTURAL USE SHALL HAVE A MOISTURE CONTENT AS SPECIFIED IN THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION."
- WOOD CONSTRUCTION SHALL CONFORM TO IBC 2004 CHAPTER 23 AND SECTION 2306 "CONVENTIONAL LIGHT -FRAME CONSTRUCTION."
- FRAMING FOR WALLS AND JOISTS SHALL BE SPRUCEPINE -FIR NO. I/NO. 2 OR BETTER. UNLESS NOTED OTHERWISE, DIMENSIONAL LUMBER REPRESENTS NOMINAL SIZES.
- SHEATHING PANELS SHALL BE MARKED WITH THE AMERICAN FLYWOOD ASSOCIATION (APA)
 TRADEMARK AND SHALL MEET THE LATEST US PRODUCT STANDARD PS I OR APA PRP -108
 PERFORMANCE STANDARDS.
- ALL WALL SHEATHING PANELS SHALL BE NOMINAL 1/2" THICK APA RATED, UNLESS
 OTHERWISE NOTED, FASTEN MITH 8D COMMON NAIL SPACED AT 6" OC AT PANEL PERIMETER
 SUPPORTED EDGES AND 12" OC AT INTERIOR INTERVIEDIATE SUPPORTS (FIELD). I -3/8" MIN.
 FASTENER PENETRATION LAY WALL NITH REQUIREMENTS OF IRC 604.
- 8. ALL ROOF SHEATHING PANELS SHALL BESYS* THICK UNLESS NOTED OTHERWISE, C -D EXTERIOR GRADE, APA RATED EXPOSURE I MEETING DOC PSI OR PS2. FASTEN WITH 8D COMMON NAILS SPACED AT 6" OC AT PANEL PERIMETER SUPPORTED EDGES AND 6" OC AT INTERIOR INTERMEDIATE SUPPORTS (FIELD). I 3% MIN. FASTENER PENETRATION, LAY ROOF SHEATHING WITH LONG DIMENSION PERPENDICULAR TO SUPPORT MEMBERS.
- 9. WOOD TO STEEL AND WOOD TO WOOD BOLTED CONNECTORS SHALL BE MADE WITH ASTM ASOT BOLTS WITH FLAT WASHERS, BOLT HOLES IN WOOD SHALL BE 1/32 LARGER THAN THE BOLT. WOOD NAILERS SHALL BE FASTENED WITH 3/6" DIA, BOLTS STAGGERED AT 20" OC UNLESS OTHERWISE NOTED.
- IO. FASTENING SCHEDULE (SEE ALSO R6023(1):
 - I. PLATE TO STUD, DIRECT: 2 16D II. STUD TO PLATE, TOENAIL: 4 - 8D
- WOOD IN CONTACT WITH SOIL, MOISTURE, WEATHER, CONCRETE, OR MASONRY SHALL BE PRESSURE TREATED SOUTHERN PINE NO. 2, OR BETTER AND APPROVED FOR THE APPLICATION.
- 14. BRACING: THE PERMANENT LATERAL BRACING SYSTEM INCLIDES PLYNOOD, WALL AND ROOF SHEATHING WITH FASTENING AND LAYOUT AS DEFINED BY: SECTION 602. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING AS REQUIRED TO LATERALLY SUPPORT THE STRICTURE DURING
- IS. ENGINEERED LIMBER (LYL, ETC.) SHALL MATCH MANUFACTURER AND SERIES LISTED OR APPROVED EQUIVALENT, PROVIDE LATERAL SUPPORT AT ALL BEARING POINTS AND ALONG COMPRESSION EDGES AT INTERVALS OF 24° OC, OR CLOSER.
- 16. MINIMUM SECTION WIDTH = 1-3/4", 3-1/2", 5-1/4" AND 7" MEMBERS MAY BE COMBINATIONS OF 1-3/4 MEMBERS, FOLLOW MANUFACTURER'S GUIDELINES FOR MULTIPLE MEMBER CONNECTIONS AND FOR SIDE LOADED BEAMS.
- IT. WOOD CONSTRUCTION CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE CO., INC., OR APPROVED EQUAL, AND INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, INCLUDING FASTENERS.
- 18. ALL FLUSH FRAMING TO HAVE APPROPRIATELY SIZED METAL JOIST HANGERS.
- LATERAL RESTRAINT REQUIRED AT ENDS OF FLOOR FRAMINGSOLID BLOCK OF SAME MATERIAL (R502.1)
- 20. BRIDGING OR CONT, IX3 BRACE NAILED TO UNDERSIDE OF FLOOR FRAMING REQUIRED AT 8' INTERVALS (R502.7.1)
- HEADERS: DEFAULT (MAX. 48" SPAN UNLESS POINT LOAD FROM ABOVE OR LATERAL BRACING REQUIREMENTS. SEE R.502.5):
 - I, INTERIOR: (2) 2X8
 - 2. EXTERIOR: (2) 2XIO (WITH 2-1/2* RIGID FOAM INSULATION).
- 22 WIND BRACING: PROVIDE DIAGONAL WIND BRACING AT ALL OUTSIDE CORNERS. AT CORNERS WITH LESS THAN 48" OF PANEL WALL, USE ALTERNATE BRACING PANELS IN ACCORDANCE WITH R60210.33. (GENERAL RETERENCE: R602)

PRE -ENGINEERED MOOD TRUSSES.

- I, ALL PRE-ENGINEERED WOOD TRUSSES SHALL CONFORM TO ANSI/TPII -2002 "NATIONAL DESIGN STANDARDS FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION."
- THE MANUFACTURER OF THE PRE -ENGINEERED TRUSSES SHALL BE A TRUSS PLATE INSTITUTE (TPI) CERTIFIED PLANT, PROOF OF CERTIFICATION SHALL BE SUBMITTED TO THE DESIGNER/ENGINEER PRIOR TO FABRICATION OF THE WOOD TRUSSES.
- 3. THE CONTRACTOR SHALL ENSURE PROPER HANDLING, BRACING, AND LATERAL RESTRAINT IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ALL TEMPORARY AND PERHANENT TRUES BRACING (INDIVIDUAL AND OVERALL) SHALL BE DESIGNED BY THE TRUSS MANUFACTURER AND INSTALLED BY THE CONTRACTOR ALL PERMANENT TRUSS BRACINGLATERAL RESTRAINT REQUIREMENTS AND LOCATIONS SHALL BE DETAILED AND SUBMITTED PRIOR TO CONSTRUCTION TO THE ENGINEER OF RECORD BY THE TRUSS MANUFACTURER. ALTERNATIVELY, THE TRUSS DESIGNER MAY DESIGN ALL TRUSSES SUCH THAT NO PERMANENT LATERAL RESTRAINT IS

- 4. ALL ROOF TRUSSES SHALL BE DESIGN FOR THE FOLLOWING UNIFORM LOADS WITH 51/2*OR 31/2*MAX BEARING, COORDINATE TRUSS BEARING WITH BEARING WALL FRAMING WIDTH:
 - A. SNOW LIVE LOAD: GROWND SNOW LOAD X 0.7= XX PSF
 - B. BOTTOM CHORD LIVE LOAD (ATTIC): 20 PSF
 - C. TOP CHORD DEAD LOAD:
- 10 PSF
- D. BOTTOM CHORD DEAD LOAD: 10 PSF
- TRUSS SHALL BE DESIGNED FOR AN UNBALANCED UNIFORM SNOW LOADING AS WELL AS ANY DRIFTED VALLEY SNOW LOADING CONDITIONS, AND WIND LOADING AS SPECIFIED IN THE PROJECT BUILDING CODE.
- 6. PRE-ENGINEERED ROOF TRUSSES TO BE APPROVED BY THE STRUCTURAL ENGINEER. TRUSS SHOP DRAMINGS SHALL BE DESIGNED, STAMPED, AND SUBMITTED BY A LICENSED PROFESSIONAL ENGINEER QUALIFIED TO PERFORM THE WORK IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMITTAL SHALL INCLUDE ALL LOADING COMBINATIONS, A FULL REPORT FOR EACH TRUSS, AND TEMPORARY AND PERMANENT LATERAL TRUSS RESTRAINT LAYOUT AND DETAILS.
- THE CONTRACTOR SHALL VERIEY THE LOCATION OF ALL VENTS, STACKS, RISERS, DRAINS, ETC. BEFORE TRISSES ARE FIXED IN PLACE.
- 8. ALL TRUSSES SHALL HAVE HURRICANE CLIPS INSTALLED AT EACH END OF EACH TRUSS IN ORDER TO PREVENT LIPT.
- 9. ALL TRUSS TO TRUSS CONNECTION DESIGNS ARE RESPONSIBILITY OF THE TRUSS MANUFACTURER. IO. ALL TEMPORARY AND PERMANENT TRUSS BRACING (INDIVIDUAL AND OVERALL) IS THE RESPONSIBILITY OF THE TRUSS DESIGNER, BRACING AND LATERAL TRUSS RESTRAINT (INCLUDING DETAILS) SHALL BE SHOWN ON TRUSS DESIGN DRAWINGS AND TRUSS ERECTION DRAWINGS.

MAGONRY

- I. CONCRETE MASONRY UNITS (CMU) SHALL BE NOMINAL THICKNESS UNLESS NOTED OTHERWISE.
- MASONRY CONSTRUCTION SHALL CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 590/ASCE 5/THS 402)
- 3. SPECIFIED MAGONRY COMPRESSIVE STRENGTH, FM = 1500PSI.
- 4. HOLLOW LOAD BEARING CMU SHALL HAVE THE FOLLOWING PROPERTIES: ASTM C40, TYPE I, GRADE N-I (NORMAL MEIGHT) WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI ACCORDING TO ASTM C40, OVEN DRY MEIGHT OVER 125PCF AND MAXIMUM MOISTURE ARCORDING TO ASTM C40, OVEN DRY MEIGHT OVER 125PCF AND MAXIMUM MOISTURE
- MORTAR SHALL BE ASTM C2TO, TYPE S WITH 28 DAY COMPRESSIVE STRENGTH OF 2000PSI. MIX MORTAR MATERIALS TO PRODUCE MORTAR CUBES HAVING A 2000PSI COMPRESSIVE STRENGTH WHEN TESTED IN ACCORDANCE WITH COMPRESSIVE STRENGTH TEST ASTM C190.
- GROUT SHALL BE ASTM C476, FINE GROUT WITH MINIMUM 26 DAY COMPRESSIVE STRENGTH OF 2000PSI.
- VERTICAL AND HORIZONTAL DEFORMED REINFORCEMENT SHALL BE ASTM A615 GR 60 AND HORIZONTAL JOINT REINFORCEMENT SHALL BE ASTM A62, GALVANIZED ACCORDING TO ASTM A641 CLASG I AS SPECIFIED.
- 8. PRISM TESTS ACCORDING TO ASTM E446 ARE REQUIRED PRIOR TO WORK.
- 4. GROUT CMU SOLID AT EXPANSION ANCHOR LOCATIONS.
- IO, CORES AND BOND BEAMS WITH REINFORCING SHALL BE FILLED SOLIDLY WITH GROUT. FILLING SUCH CORES AND BOND BEAMS WITH MORTAR IS STRICTLY PROHIBITED. IN ADDITION, CARE SHALL BE EXERCISED IN KEEPING CORES FREE FROM MORTAR DROPPINGS.
- II. MINIMUM REINFORCING REQUIREMENTS FOR REINFORCED CMU WALLS SHALL CONFORM TO THE SCHEDULE SHOWN ON THE CONTRACT DRAWINGS AND THE APPLICABLE BUILDING CODE REQUIREMENTS.
- 12. GROUT SHALL BE PLACED USING LOW OR HIGH LIFT GROUTING PROCEDURES CONFORMING TO ACUAGCE. TERMINATE GROUT POURS I-I/2" BELOW TOP COURSE OF PLACEMENT. REINFORCING SHALL BE SPLICED A MINIMUM OF 40 BAR DIAMETERS.
- 13. VERTICAL REINFORCING SHALL BE SECURELY HELD IN PROPER ALIGNMENT AND POSITION DIRING GROUTING OPERATIONS BY USING "REBNAR POSITIONERS," AS MANUFACTURED BY WIRE BOND OR APPROVED EQUIVALENT, THE PRODUCT, IN ADDITION, SHALL ALLOW FOR GUIDING THE SPLICED REINFORCING DROPPED FROM THE TOP OF THE LIFT.
- 14. MASONRY SHALL BE BRACED DURING CONSTRUCTION. BRACE SPACING SHALL NOT EXCEED TEN TIMES THE WALL THICKNESS BUT NOT LESS THAN THE PROCEDURES LISTED UNDER NOMA-TEK 72
- 15. PROVIDE FULL HEIGHT VERTICAL REINFORCEMENT AT EACH SIDE OF CONTROL JOINTS, WINDOWS, DOORS, AND WALL OPENINGS, AT ALL ENDS OF WALLS AND CORNERS. REINFORCING SHALL BE GROUTED SOLID AND MATCH THE DIAMETER OF THE TYPICAL WALL REINFORCING.

PIRE RESISTANT CONSTRUCTION

- FOLLOW SECTION 302. A FEW COMMON CRITICAL LOCATIONS FOLLOW:
- A. GARAGE/RESIDENCE OR GARAGE/ATTIC SEPARATION 5/8" TYPE X GYPSM DRYWALL AT GARAGE SIDE WHEN ADJACENT TO LIVING SPACE. 5/8" TYPE X DRYWALL REQUIRED AT CEILING WHEN LIVING SPACE ABOVE. (TABLE R302.6)
- B. ENCLOSED ACCESSIBLE SPACE UNDER STAIRS REQUIRES MIN. " GYPSUM (R302.7)
- C. FIREBLOCKING IS REQUIRED TO ISOLATE EACH FLOOR LEVEL. 2X BLOCKING AND "
 GYPSIM AND FIBERGLASS/MINERAL WOOL IF SECURE ARE ALL ACCEPTABLE
 (R302.II.I)
- 2. DUPLEX/2 FAMILITANDARD SEPARATION IS 5/8" TYPE X BOTH SIDES. (R302.3)



Alternative Designs Inc.

Residential/Commercia Design 94 Granite Street Manchester, NH 03101 phone: (603) 645-4388 fax: (603) 645-6010

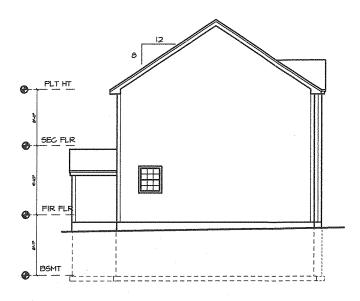
contractor to check & verify all selector to check & verify all selector construction. Il construction shall be in strict ampliance with The State of selector to the National selector of Massachusetts shallon Codes, whicheve applica

REVISIONS

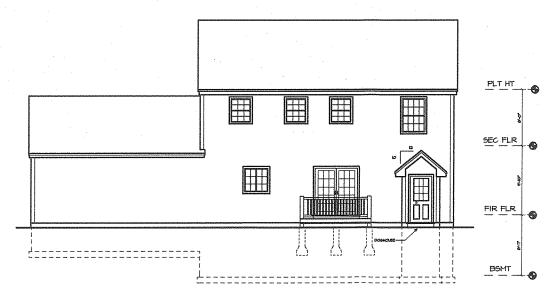
	14-044
F	EB 2014

SHEET 2 OF 8









REAR ELEVATION

A3 SCALE: 1/8" = 1'-0"

		MINDOW SCHEDULE			
MARK	QTY	MODEL NUMBER	R50	NOTES	
Α		244DH3049		DBL HUNG	
В		244DH2836		DBL. HUNG	
С		CN235		CASEMENT	

- I. RSO TO BE DETERMINED BY WINDOW MANUFACTURER.

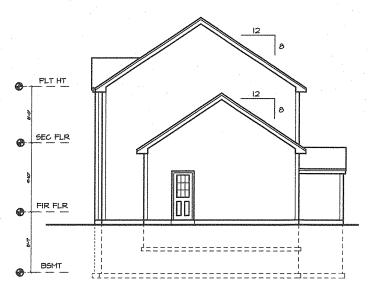
 2. BEDROOM WINDOWS TO MEET EGRESS

 3. IN ACCORDANCE WITH I.R.C.(2009)-R&I22, WHERE THE OPENING OF AN OPERABLE WINDOW IS MORE THAN 12" ABOVE THE EXT. FINISHED GRADE OR EXT. DECK BELOW, THE LOWEST PART OF THE CLEAR OPENING IS TO BE A MIN, OF 24" ABV. THE FIN, FLR.

 4. WINDOWS ARE BASED ON ANDERSEN 200 SERIES TILT-WASH MODEL NUMBERS

DOOR SCHEDULE							
MARK	aty	SIZE	R50	NOTES			
1		3'0 × 6'8		EXT. DOOR W SIDE LIGHTS			
2		2'8 × 6'8		4-LITE			
3		2'6 × 6'8	-	INTERIOR			
4		2'4 × 6'8	·	INTERIOR			
5		6'0 × 6'8		BIFOLD			
6		5'0 × 6'8		BIF <i>O</i> LD			
7.		4'0 × 6'8		BIFOLD			
В		6'0 × 6'8		EXT. SLIDER DOOR			
9		2'8 × 6'6		STEEL INSUL.			
10		1'2 × 6'8		INTERIOR			
11		2'8 × 6'8		FIRERATED DOOR			
12		1'4 × 6'8	·	INTERIOR			

RSO TO BE DETERMINED BY DOOR MANUFACTURER.



RIGHT SIDE ELEVATION

SCALE: 1/8" = 1'-0"



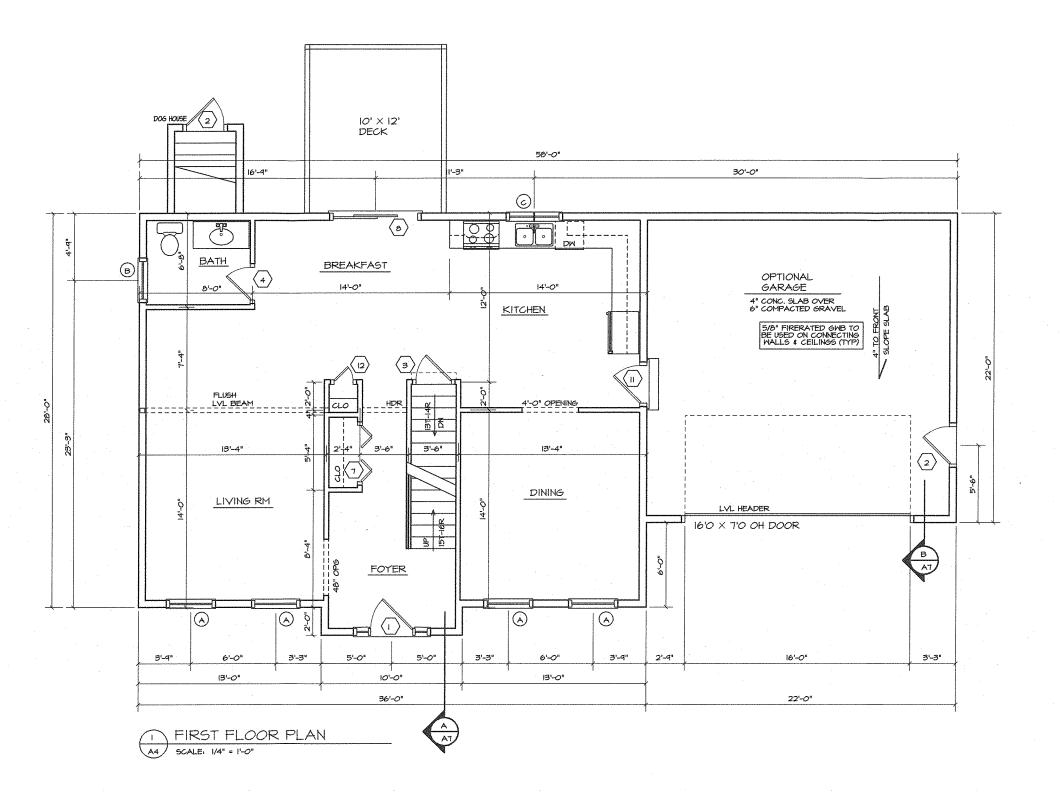
Designs Inc. www.altdesigns.us lesidential/Commercial

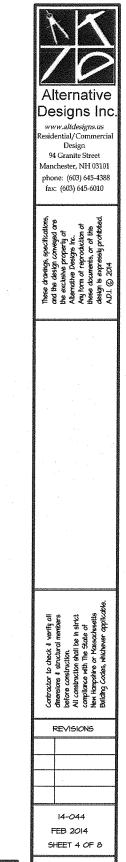
Design 94 Granite Street Manchester, NH 03101 phone: (603) 645-4388

fax: (603) 645-6010

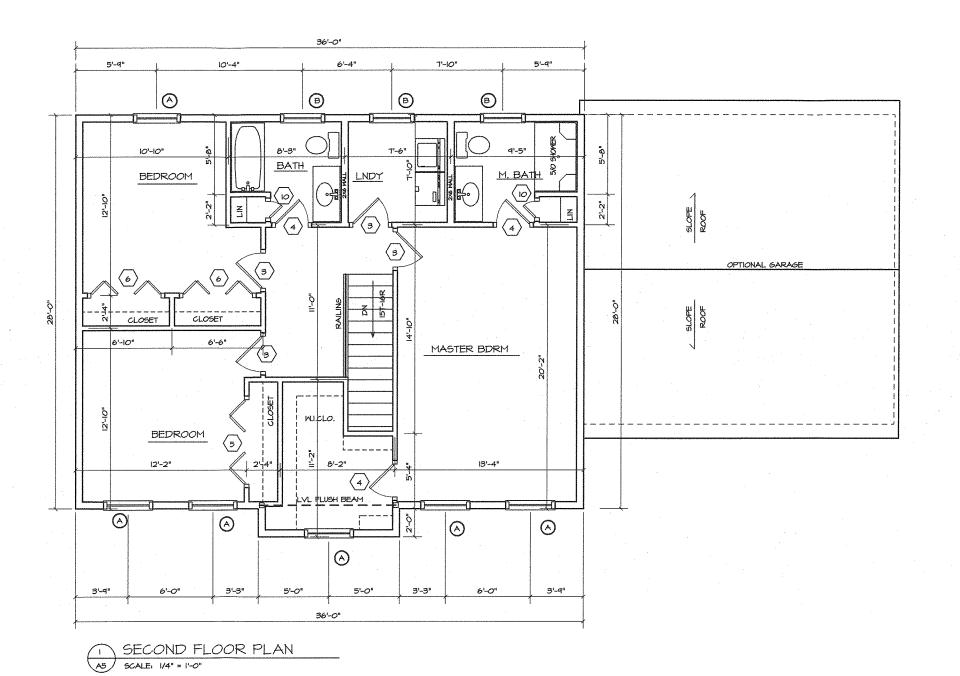
REVISIONS

14-044 FEB 2014 SHEET 3 OF 8





Alternative Designs Inc





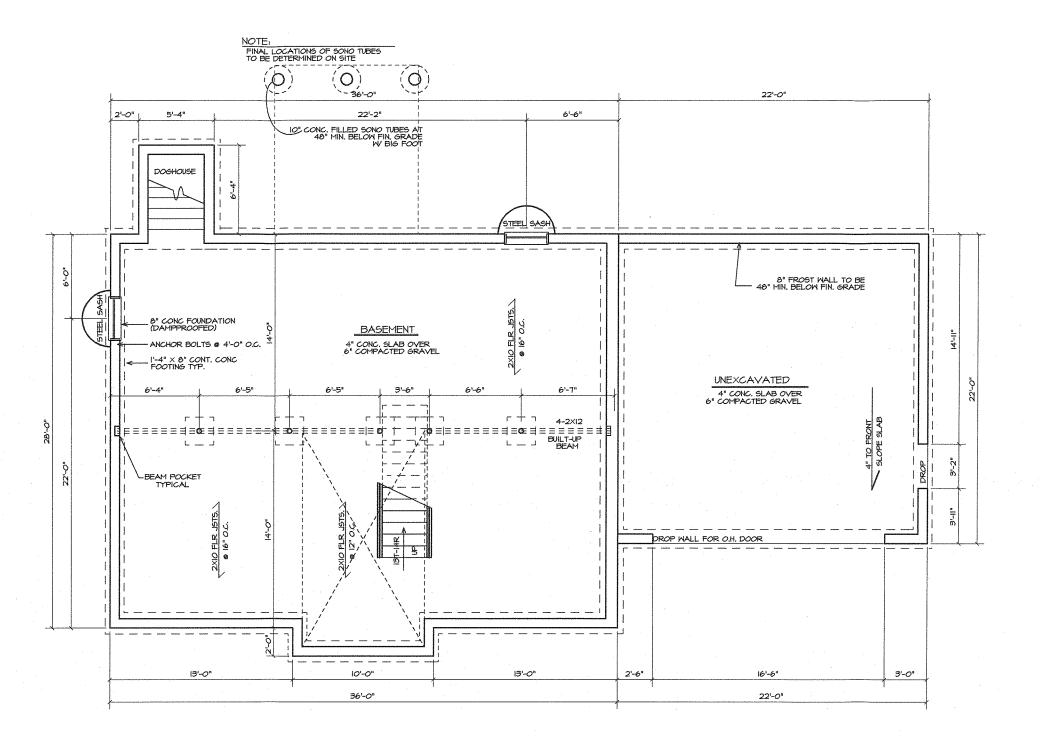
REVISIONS

14-044 FEB 2014 SHEET 5 OF 8

A5

Alternative Designs Inc.

14-044



FOUNDATION PLAN

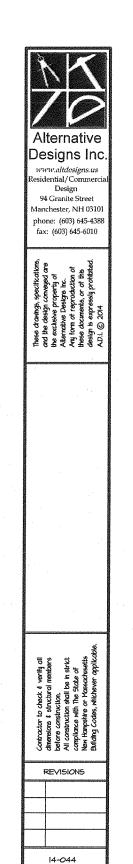
6 SCALE: 1/4" = 1'-0"

GENERAL NOTES

CONC BULKHEAD SIZE AND LOCATION TO BE DETERMINED BY SITE CONDITIONS AND/OR CONTRACTOR

WALKOUTS AS PER SITE CONDITIONS AND CONTRACTOR

STEEL SASH WINDOW SIZES AND LOCATIONS TO BE DETERMINED BY CONTRACTOR



FEB 2014 SHEET 6 OF 8

GENERAL NOTES

- A. 2XI2 RIDGE BOARD
- B. 2X IO RAFTERS 16" O/C (UNLESS OTHERWISE NOTED)
 PLYWOOD SHEATHING, 15# BUILDING PAPER, & 235# ASPHALT
 SHINGLES W/ ICE SHIELD AT RAFTER TAILS AND VALLEYS.
- C. 2X6 COLLAR TIES AT 32" O/C (TYPICAL)
- D. 2x8 CEILING JOISTS AT 16" O/C WITH R-38 FIBERGLASS BATT INSULATION (TYPICAL)
- E. METAL DRIP EDGE, IX4 PINE BLOCKING (SUB-FASCIA) IX8 PINE BOARD FASCIA, & 3/8" EXTERIOR, AC PLYWOOD SOFFIT WITH 2" CONTINUOUS LOUVERED VENTS (TYPICAL)
- F. 2x6 STUDS 16" O/C, R-21 FIBERGLASS BATT INSULATION IN BETWEEN, 1/2" PLYWOOD SHEATHING & EXTERIOR SIDING W "TYVEK" OR EGUAL (OPTIONAL) AND 1/2" GYP. BD. ON THE INTERIOR.

SECTION SCALE: 1/4" = 1'-0"

- 6. 2-2×6 TOP PLATES AND I-2×6 SHOE (BOTTOM PLATE)
- H. 2XIO FLOOR LOISTS 16" O/C (UNLESS OTHERWISE NOTED)
 WITH 3/4" T&G SUBFLOOR (GLUED & NAILED) R-30 FIBERGLASS
 BATT INSULATION AT FIRST FLOOR ONLY.

- J. 6" CONCRETE FOUNDATION WALL WITH 1-2X6 PRESSURE TREATED SILL PLATE W SILL SEALER; ANCHOR BOLTS @ 4"-0" O.C. (TYPICAL)
- K. 4" CONCRETE SLAB FLOOR OVER (MIN. 6") COMPACTED GRAVEL
- L. 8 " CONCRETE PROST WALL TO BE 48" MIN. BELOW FINISHED GRADE
- M. 1'-4" X 8" CONTINUOUS CONCRETE FOOTING (TYPICAL)
- N. 4-2XI2 BUILT-UP BEAM OVER 3 I/2" DIAM. STEEL LALLY COLUMN WITH TOP AND BOTTOM END PLATES, OVER 24"X24"XI2" CONCRETE FOOTINGS.
- O. IX3 STRAPPING AT 16" O/C & 1/2" GYP. BD. (TYPICAL)
- P. 3-2XI2 STAIR STRINGERS
- Q. CONTINUOUS RIDGE VENT
- R. 2X6 STUD WALL @ 16" O.C.
- S. HURRICANE CLIPS AND FRAMING ANCHORS AS REQ'D.
- T. 2" RIGID INSULATION INSIDE FACE OF CONCRETE WALL TO TOP OF SLAB

DESIGN LOADS

LIVE LOAD AT LIVING SPACES: 40 PSF
LIVE LOAD AT SLEEPING SPACES: 30 PSF
GROUND SHOW LOAD; 60 PSF

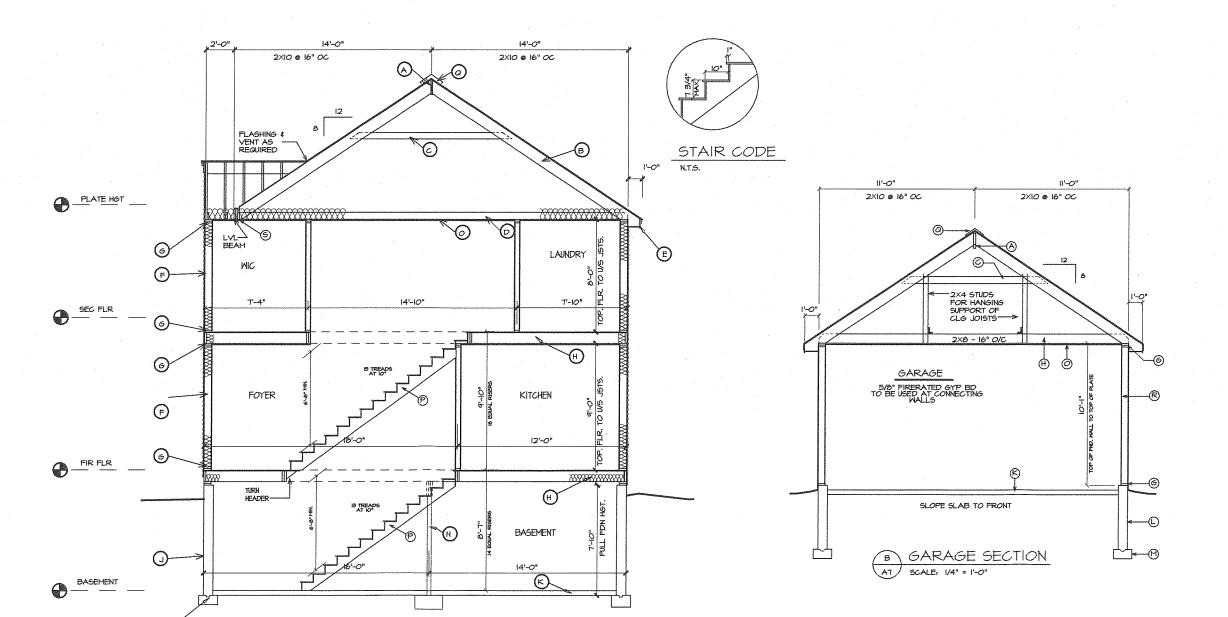
FRAMER TO INSTALL DOUBLE FLOOR JOISTS UNDER PARTITION WALLS PARALLEL TO JOIST DIRECTION.

PROVIDE IX4 CROSS BRIDGING AT MID POINT OF SPAN OR 8'-O" O.C. MAXIMUM IN ALL FLOORS.

WHERE PREENGINEERED FLOOR OR ROOF TRUSSES ARE USED, TRUSS MANUFACTURER MUST PROVIDE SHOP DRAWINGS WHICH BEAR SEAL OF REGISTERED ENGINEER IN STATE IN WHICH WORK IS TO BE PERFORMED.

ALL LUMBER MUST BE NO. 2 OR BETTER, SPRUCE - PINE - FIR.

PROVIDE MOISTURE VAPOR RETARDERS IN ALL FRAMED WALLS, FLOORS AND ROOF/CEILINGS IN ACCORDANCE WITH I.R.C. SECTIONS R-506.2.3 AND R601.3





Alternative
Designs Inc.

www.altdesigns.us

Design 94 Granite Street Manchester, NH 03101 phone: (603) 645-4388 fax: (603) 645-6010

esidential/Comme

These drawings, specifications, and the design conveyed are the exclusive property of Alternative Designs Inc.. Any form of reproduction of these documents, or of this design is expressly prohibited design is expressly prohibited.

contractor to check & verify all lineasions & structural members efore construction. Al construction shall be in strict ompliance with The State of

REVISIONS

14-044 FEB 2014 SHEET 7 0F8

A7

